

APPENDIX A

HYDROLOGIC CALCULATIONS

HYDROLOGIC CALCULATIONS

Hydrologic analyses were conducted to determine peak discharges from a 100-year and a 500-year storm event for the Beartrap Creek, Mike Horse Creek, and Upper Blackfoot River drainages. Preliminary peak flow estimates were calculated using methods provided in WRI-03-4308 (Parrett, 2004). Using the regression equation for the Northwest Region of Montana, a mean annual precipitation of 18 inches (NOAA Rogers Pass weather station), and drainage basin areas, peak flows for each drainage area were calculated. Based upon the USGS regression equation, the estimated peak flows are:

Drainage Basin	Drainage Basin Area (mi ²)	100-Year Peak Flow (cfs)	500-Year Peak Flow (cfs)
Beartrap Creek	2.02	297.8	766.3
Mike Horse Creek	0.41	96.0	261.6
Upper Blackfoot River	9.9	920.6	2237.1

The Beartrap Creek Analysis applies to both the portion of Beartrap to be re-established through the Mike Horse Tailings Impoundment area, and the portion of the Creek between the impoundment and the confluence with Anaconda Creek.

An analysis was also conducted to determine preliminary estimates of floodplain dimensions for the portions of Beartrap Creek and Mike Horse Creek drainages included in the EE/CA (A similar analysis will be performed for the Upper Blackfoot River once a detailed topographic survey is completed). Depth-Duration Factors, which define the distribution of precipitation as a function of time, were obtained from WRI-97-4004 (Parrett, 1997). Once the magnitude and distribution of the 100-year and 500-year storm events with respect to time were determined, the timing and volume of resulting runoff could be determined using the HEC-1 rainfall-runoff model (Corps of Engineers, 1988). In addition to the precipitation hyetograph, new information used in conjunction with the HEC-1 model included:

- Soil-Cover Complex Curve Number of 84;
- Time of Concentration of 0.38 hours (USGS, 1996); and
- Basin Storage Coefficient of 3.27 hours (USGS, 1996).

Using the Clark Unit Hydrograph, the peak flow from the 100-yr storm is 281 cubic-feet-per-second (cfs), and the peak flow from the 500-yr storm is 486 cfs. These are similar to the results obtained using the USGS regression equation in WRI 92-4048. HEC-1 also provided estimates of flow depths, which were used to estimate the floodplain widths for both the 100-year and 500-year storm. Floodplain widths for Beartrap Creek ranged from approximately 45 feet to 25 feet, and a value of 30 feet was used in the EE/CA preliminary design.

Hydraulic calculations were also performed to determine average riprap sizes required for stream channel design for portions of Beartrap Creek and Lower Mike Horse Creek for certain removal action options (engineered channel with riprap is not proposed for the Upper Blackfoot River). Using Flowmaster® software and assuming a roughness coefficient of 0.040, channel depths between 1 and 6 feet, depending on bottom width, flow velocities were

determined for the 100-year and 500-year storms (Haestad Methods, 1994). Typical channel dimensions, velocities and average riprap dimensions are shown in the following table. More detailed analyses of channel design and riprap sizing will be completed during final design of the selected site-wide alternative.

Location	Channel Width (ft)	Channel Depth (ft)	Avg. Velocity (fps)	Avg. Riprap Size (in)*
Beartrap Creek	8	2.2-3.4	9.5-12.2	8-12
Lower Mike Horse Creek	5	1.2-1.9	9.6-12.6	8-12
Upper Blackfoot River	15	3.7-5.7	9.7-12.3	8-12

*Source: Federal Highway Administration (FHWA) 1967, Nomograph for determining the stone size based on velocity and side slope from Richardson and Julien, 1990.

References

Corps of Engineers, 1988. HEC-1 Flood Hydrograph Package.

Haestad Methods, 1994. Flowmaster Open Channel and Pipe Hydraulic Analysis Software version 5.13, Waterbury, CT.

Parrett, C., 1997. Regression Equations For Mean Values Of Annual Precipitation Maxima Region 1, USGS WRI-97-4004.

Parrett, C., Johnson, D.R., 2004. Methods for Estimating Flood Frequency in Montana Based on Data Through Water Year 1998, USGS WRI-03-4308.

Richardson, E.V., D.B. Simons, and P.Y. Julien, 1990. Highways in the River Environment, Federal Highway Administration.

U.S.G.S., 1996. Estimation of Unit Hydrographs For Large Floods at Ungaged Sites in Montana.

REGRESSION EQUATIONS FOR MEAN VALUES OF ANNUAL PRECIPITATION MAXIMA REGION 1¹

Duration Equation

24-hr	Mean = 1.4 +(0.18 x LAT) - (0.13 x LONG) + (0.019 x MAP)
6-hr	Mean = 0.75 +(0.87 x LAT) - (0.041 x LONG)
2-hr	Mean = 0.69 +(0.034 x LAT) - (0.029 x LONG)

LAT = 7.0248
 LONG = 12.3536
 MAP = 18 in

24-hr = 1.40 inches
 6-hr = 0.85 inches
 2-hr = 0.57 inches

RETURN FREQUENCY MEAN VALUE MULTIPLIERS FOR REGION 2²

DURATION	100	500 years
24 -HR	2.57	3.40
6-HR	2.59	3.50
2-HR	2.78	3.90

RETURN FREQUENCY (years) PRECIPITATION AMOUNTS (inches)

	100	500 years
24 -HR	3.60	4.76
12-HR	2.86	3.82
6-HR	2.21	2.99
3-HR	1.88	2.58
2-HR	1.59	2.23
1-HR	1.38	1.97
15-MIN	0.78	1.12
5-MIN	0.23	0.33

EQUATIONS USED IN ABOVE TABLE

- 24-HR = 24-HR average maxima x mean value multiplier
- 12-HR = .467 x 24-HR + .533 x 6-HR
- 6-HR = 6-HR average maxima x mean value multiplier
- 3-HR = .467 x 6-HR + .533 x 1-HR
- 2-HR = 2-HR average maxima x mean value multiplier
- 1-HR = (2-HR - 0.25 x 6-HR) / .75
- 15-MIN = 0.57 x 1-HR
- 5-MIN = 0.29 x 1-HR

¹ Table 11, USGS WRI 97-4004

² Figure 12, USGS WRI 97-4004

ESTIMATE OF PEAK FLOW FOR BEAR TRAP CREEK

REGIONAL FLOOD -FREQUENCY EQUATIONS BASED ON DRAINAGE-BASIN CHARACTERISTICS*

$$Q_{100} = 56.4 \cdot A^{0.71} \cdot P^{0.403}$$

Northwest Region

$$Q_{500} = 175 \cdot A^{0.674} \cdot P^{0.347}$$

Northwest Region

A = DRAINAGE AREA (mi²)

P = MEAN ANNUAL PRECIPITATION (in)

A = 2.02 mi²

P = 18 in FROM NOAA ROGERS PASS INFORMATION

$$Q_{100} = 297.8 \text{ cfs}$$

$$Q_{500} = 766.3 \text{ cfs}$$

*Parrett, C, and Johnson, D.R., 2004, Methods for Estimating Flood Frequency in Montana Based on Data Through Water Year 1998, USGS WRI-03-4308.

ESTIMATE OF PEAK FLOW FOR MIKE HORSE CREEK

REGIONAL FLOOD -FREQUENCY EQUATIONS BASED ON DRAINAGE-BASIN CHARACTERISTICS*

$$Q_{100} = 56.4 \cdot A^{0.71} \cdot P^{0.403}$$

Northwest Region

$$Q_{500} = 175 \cdot A^{0.674} \cdot P^{0.347}$$

Northwest Region

A = DRAINAGE AREA (mi²)

P = MEAN ANNUAL PRECIPITATION (in)

A = 0.41 mi²

P = 18 in FROM NOAA ROGERS PASS INFORMATION

$$Q_{100} = 96.0 \text{ cfs}$$

$$Q_{500} = 261.6 \text{ cfs}$$

*Parrett, C, and Johnson, D.R., 2004, Methods for Estimating Flood Frequency in Montana Based on Data Through Water Year 1998, USGS WRI-03-4308.

ESTIMATE OF PEAK FLOW FOR UPPER BLACKFOOT RIVER

REGIONAL FLOOD -FREQUENCY EQUATIONS BASED ON DRAINAGE-BASIN CHARACTERISTICS*

$$Q_{100} = 56.4 * A^{0.71} * P^{0.403}$$

Northwest Region

$$Q_{500} = 175 * A^{0.674} * P^{0.347}$$

Northwest Region

A = DRAINAGE AREA (mi²)

P = MEAN ANNUAL PRECIPITATION (in)

A = 9.9 mi²

P = 18 in FROM NOAA ROGERS PASS INFORMATION

$$Q_{100} = 920.6 \text{ cfs}$$

$$Q_{500} = 2237.1 \text{ cfs}$$

*Parrett, C, and Johnson, D.R., 2004, Methods for Estimating Flood Frequency in Montana Based on Data Through Water Year 1998, USGS WRI-03-4308.

Computation for Lag Time

$$A = 1.98 \text{ mi}^2$$

$$TC = 0.29 * A^{0.65}$$

$$R = 2.90 * A^{0.31}$$

$$T_c = 0.452096 \text{ use } 20 \text{ min}$$

$$R = 3.583969 \text{ use } 4$$

BRTRAP100.OUT

```
1*****  
* * FLOOD HYDROGRAPH PACKAGE (HEC-1) *  
* * JUN 1998 *  
* * VERSION 4.1 *  
* * RUN DATE 05JUN06 TIME 09:57:16 *  
* *  
*****
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```
      X   X   XXXXXXXX  
      X   X   X   X  
      X   X   X   X  
XXXXXX XXXX XXXXXX  
X   X   X   X   X  
X   X   X   X   X  
X   X   X   X   X  
X   X   X   X   XXXX
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTTOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLED INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

PAGE 1

HEC-1 INPUT

```
1 LINE ID. ....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
1 ID BEAR TRAP CREEK  
2 ID 100 YEAR FLOODPLAIN ASSESSMENT  
3 ID ROUTING USING HEC-1  
4 ID M. RHODES, HYDROMETRICS INC.  
5 ID 13 MAR06  
* *** TIME SPECIFICATION  
6 IT 2 08APR03 1200 300  
* *** GLOBAL OUTPUT OPTIONS  
7 IO 4  
* *** 100 YR HYETOGRAPH  
*  
8 KK P 100  
9 KM PRECIP  
10 BA 1.98  
11 PH 0.00 0.23 0.78 1.38 1.59 1.88 2.21 2.86 3.60  
* *** INFLOW TO CREEK  
* *** CLARK'S UNIT HYDROGRAPH  
12 UC 0.33 2.88  
13 LS 0 84
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Page 1

*
* ***
*

14
15 KK REACH1
KM BEAR TRAP CREEK BEGIN TO 25+00 FEET DOWNSTREAM
RD 0.08 0.07 0.08 2500.0 .208
RC 0.0 100.0 200.0 203 .205
RX 0.0 6340 6300 6298 6300 300 400 500
RY 6360 6340 6300 6298 6300 6315 6325 6370
* * ***
*

20
21 KK REACH2
KM CHANNEL ROUTING 25+00 TO 40+00
RD 0.08 0.07 0.08 1500 .133
RC 0.0 100 200 280 .283
RX 0.0 5680 5620 5600 5597 286 300 400
RY 5720 5680 5620 5600 5600 5610 5610 5700
* * ***
*

26 KK REACH3
KM CHANNEL ROUTING 40+00 TO 59+00
RD 0.08 0.07 0.08 1900 .060
RC 0.0 40.0 44.0 48.0 60.0 120 140 160
RX 0.0 5486 5483 5486 5488 5489 5490 5491
RY 5494 5486 5483 5486 5488 5489 5490 5491
* * ***
*

1 LINE ID.1.....2.....3.....4.....5.....6.....7.....8.....9.....10
HEC-1 INPUT

32 KK REACH4
KM CHANNEL ROUTING 59+00 TO 68+00
RD 0.08 0.07 0.08 900 .030
RC 0.0 40.0 80.0 120 138 140 200 320
RX 0.0 5467 5460 5453 5449 5443 5441 5465
RY 5449 5460 5453 5449 5443 5441 5465 5500
* * ***
*

38 KK REACH5
KM CHANNEL ROUTING 68+00 TO 78+00
RD 0.08 0.07 0.08 1000 .030
RC 0.0 40.0 80.0 136 140 144 180 240
RX 0.0 5432 5429 5425 5422 5419 5422 5437
RY 5429 5425 5422 5419 5422 5437 5460
* * ***
*

44 KK REACH6
KM CHANNEL ROUTING 78+00 TO 92+00
RD 0.08 0.07 0.08 1400 .030
RC 0.0 20.0 37.0 40.0 44.0 64 100 120
RX 0.0 20.0 37.0 40.0 44.0 64 100 120
Page 2

7	TO	OUTPUT CONTROL VARIABLES	
		IOPRT	4 PRINT CONTROL
		IPILOT	0 PLOT CONTROL
		QSCAL	0. HYDROGRAPH PLOT SCALE
		HYDROGRAPH TIME DATA	
		NMIN	2 MINUTES IN COMPUTATION INTERVAL
		IDATE	8APR 3 STARTING DATE
		ITIME	1200 STARTING TIME
		NQ	300 NUMBER OF HYDROGRAPH ORDINATES
		NDDATE	8APR 3 ENDING DATE
		NDTIME	2158 ENDING TIME
		ICENT	.19 CENTURY MARK
		COMPUTATION INTERVAL	
		TOTAL TIME	.03 HOURS
		BASE	9.97 HOURS
		ENGLISH UNITS	
		DRAINAGE AREA	SQUARE MILES
		PRECIPITATION DEPTH	INCHES
		LENGTH, ELEVATION	FEET
		FLOW	CUBIC FEET PER SECOND
		STORAGE VOLUME	ACRE FEET
		SURFACE AREA	DEGREES FAHRENHEIT
		TEMPERATURE	

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*          *
*          P 100
*          *
*****          DRIFTCD
8   KK

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SUBBASIN RUNOFF DATA

10 BA SUBBASIN CHARACTERISTICS
TAREA 1.98 SUBBASIN AREA

PRECIPITATION DATA

11 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
HYDRO-35 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
5-MIN 15-MIN 60-MIN
.23 .78 1.38 1.59 1.88 2.21 2.86 3.60 .00 .00 .00

13 LS SCS LOSS RATE

STRTL CRVNBR RTIMP .38 84.00 .00 INITIAL ABSTRACTION
CURVE NUMBER PERCENT IMPERVIOUS AREA

12 UC CLARK UNITGRAPH

TC R .33 2.88 TIME OF CONCENTRATION
TIME STORAGE COEFFICIENT

SYNTHETIC ACCUMULATED-AREA VS. TIME CURVE WILL BE USED

CLARK UNIT HYDROGRAPH PARAMETERS
SNYDER TC=.33 HR, R= 2.88 HR
TP=.35 HR, CP=.11

300 END-OF-PERIOD ORDINATES

10.	38.	80.	131.	189.	250.	307.	354.	391.
		407.	402.	397.	393.	388.	384.	379.
		366.	362.	358.	354.	350.	346.	342.
		330.	323.	319.	315.	312.	308.	305.
		294.	291.	287.	284.	281.	278.	274.
		262.	259.	256.	253.	250.	247.	244.
		233.	231.	228.	225.	223.	220.	218.
		208.	205.	203.	201.	198.	196.	194.
		185.	183.	181.	179.	177.	175.	173.
		165.	163.	161.	159.	157.	156.	154.
		147.	145.	144.	142.	140.	139.	137.
		131.	129.	128.	126.	125.	123.	122.
		117.	115.	114.	113.	111.	110.	109.
		104.	103.	101.	99.	98.	97.	96.
		92.	91.	89.	88.	87.	86.	85.
		82.	81.	80.	79.	78.	77.	76.
		73.	72.	71.	70.	69.	68.	67.
		65.	64.	63.	62.	61.	60.	59.
		58.	58.	56.	55.	54.	53.	52.
		52.	51.	50.	49.	48.	47.	47.
		46.	46.	45.	44.	43.	42.	42.
		41.	41.	40.	39.	38.	37.	37.
		37.	36.	35.	35.	34.	33.	33.
		33.	32.	32.	31.	30.	30.	29.
		29.	29.	28.	28.	27.	26.	26.
		26.	26.	25.	25.	24.	24.	24.
		23.	23.	22.	22.	22.	21.	21.
		21.	20.	20.	20.	19.	19.	19.
		18.	18.	18.	17.	17.	17.	17.

BRTRAP100.OUT

ANR .080 RIGHT OVERBANK N-VALUE
RLNTH 1500. REACH LENGTH
SEL .1330 ENERGY SLOPE
ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

25 RY ELEVATION --- LEFT OVERBANK ---+----- MAIN CHANNEL ---+--- RIGHT OVERBANK ---
24 RX DISTANCE 5720.00 5680.00 5620.00 5600.00 5597.00 5600.00 5610.00 5700.00
.00 100.00 200.00 280.00 283.00 286.00 300.00 400.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	2.15	11.56	28.57	52.61	81.18	113.75	150.33	190.93	235.53
OUTFLOW	.00	816.31	7552.93	25360.65	60857.84	116845.60	190205.90	281548.50	391588.20	521092.70
ELLEVATION	5597.00	5603.47	5609.95	5616.42	5622.89	5629.37	5635.84	5642.32	5648.79	5655.26

STORAGE	284.13	336.75	393.38	454.03	519.49	590.16	666.04	746.19	829.94	917.31
OUTFLOW	670855.10	841680.40	1034376.00	1248901.00	1484287.00	1746382.00	2038086.00	2376225.00	2743994.00	3141973.00
ELLEVATION	5661.74	5668.21	5674.68	5681.16	5687.63	5699.10	5700.58	5707.05	5713.53	5720.00

* *

* REACH3 *

* *****

CHANNEL ROUTING 40+00 TO 59+00

HYDROGRAPH ROUTING DATA

28 RD MUSKINGUM-CUNGE CHANNEL ROUTING

29 RC NORMAL DEPTH CHANNEL
ANL .080 LEFT OVERBANK N-VALUE
ANCH .070 MAIN CHANNEL N-VALUE
ANR .080 RIGHT OVERBANK N-VALUE
RLNTH 1900. REACH LENGTH
SEL .0600 ENERGY SLOPE
ELMAX .0 MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

30 RY ELEVATION --- LEFT OVERBANK ---+----- MAIN CHANNEL ---+--- RIGHT OVERBANK ---
30 RX DISTANCE 5494.00 5486.00 5483.00 5486.00 5488.00 5489.00 5490.00 5491.00
.00 40.00 44.00 48.00 60.00 120.00 140.00 160.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.02	.08	.18	.31	.49	.74	1.16	1.73	2.52
OUTFLOW	.00	5483.00	5483.58	5484.16	5484.74	5485.32	5485.90	5486.47	5487.05	5487.63
ELLEVATION	5483.00	5483.58	5484.16	5484.74	5485.32	5485.90	5486.47	5487.05	5487.63	5488.21

STORAGE	4.10	6.52	9.33	12.51	16.06	19.77	23.55	27.41	31.35	35.35
OUTFLOW	533.61	987.80	1664.42	2530.10	3559.11	4895.64	6381.14	8047.70	9889.78	11903.82
ELLEVATION	5488.79	5489.37	5489.95	5490.53	5491.11	5491.69	5492.27	5492.84	5493.42	5494.00

CHANNEL ROUTING 59+00 TO 68+00

HYDROGRAPH ROUTING DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

NORMAL DEPTH CHANNEL				
ANL	.080	LEFT OVERBANK N-VALUE		
ANCH	.070	MAIN CHANNEL N-VALUE		
ANR	.080	RIGHT OVERBANK N-VALUE		
RLNTH	900.	REACH LENGTH		
SEL	.0300	ENERGY SLOPE		
ELMAX	.0	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION		

CROSS-SECTION DATA					
	LEFT	OVERBANK	---	MAIN CHANNEL	---
ELEVATION	5467.00	5460.00	5453.00	5449.00	5443.00
DISTANCE	.00	40.00	80.00	120.00	138.00
					140.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

NORMAL	DEPTH	CHANNEL	
ANL	.080	LEFT	OVERBANK N-VALUE
ANCH	.070	MARIN CHANNEL N-VALUE	

* * * * *

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BRTRAP100.OUT

ANR	.080	RIGHT OVERBANK	N-VALUE
LNLNTH	1.000	REACH LENGTH	
SEL	.0300	ENERGY SLOPE	
ELMAX	.0	MAX. ELEV. FOR STORAGE	

	3 RY	3 RX	MAIN CHANNEL	---	RIGHT OVERBANK	---
--- LEFT OVERBANK	54.32	0.00	54.22	0.00	54.37	0.00
--- DISTANCE	40.00	40.00	54.25	0.00	54.22	0.00
--- ELEVATION	2.2	2.00	54.25	0.00	54.19	0.00
			54.22	0.00	54.22	0.00
			1.36	0.00	1.40	0.00
					1.44	0.00
					1.80	0.00
					2.40	0.00

**

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

	STORAGE	OUTFLOW	OUTFLOW									
	.00	.14	.75	.00	.16	.16	.90	.60	.58	.90	.60	.58
OUTFLOW	.00	.20	.54	.00	.54	.54	.40	.15	.54	.40	.15	.54
ELEVATION												
STORAGE	.00	.14	.75	.00	.16	.16	.90	.60	.58	.90	.60	.58
OUTFLOW	.00	.20	.54	.00	.54	.54	.40	.15	.54	.40	.15	.54
ELEVATION	54.19	54.21	54.21	54.19	54.21	54.21	54.23	54.23	54.23	54.21	54.21	54.21
STORAGE	56.90	66.42	76.21	86.29	96.64	107.28	118.19	129.38	140.85	152.60	167.00	182.00
OUTFLOW	48.98	61.15	75.00	91.06	117.96	126.80	145.80	166.30	189.07	212.73	233.70	254.60
ELEVATION	54.40	54.42	54.42	54.44	54.44	54.47	54.49	54.51	54.53	54.55	54.57	54.60

HYDROGRAPH ROUTING DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

DEPTH	CHANNEL	LEFT	OVERBANK	N-VALUE
ANL	.080			
ANCH	.070	MAIN	CHANNEL	N-VALUE
ANR	.080	RIGHT	OVERBANK	N-VALUE
RLNTH	1400.		REACH LENGTH	
SEL	.0300	ENERGY SLOPE		
FITMAX	0	MAX FLOW	FOR STORAGE / OUTFLOW	CALCULATION

CROSS-SECTION DATA						
ELEVATION	LEFT DISTANCE	OVERBANK	MAIN CHANNEL	---	---	RIGHT OVERBANK
5416.00	5412.00	5410.00	5408.00	5409.00	5410.00	5414.00
	20.00	37.00	40.00	44.00	64.00	120.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA						
STORAGE	.00	.02	.06	.16	.37	.71
OUTFLOW	.00	.60	3.84	10.26	28.66	68.60
ELEVATION	5408.00	5408.42	5408.84	5409.26	5409.68	5411.10
STORAGE	4.60	5.76	7.02	8.36	9.78	11.27
OUTFLOW	807.73	1092.07	1422.43	1799.82	2225.43	2727.41
ELEVATION	5412.21	5412.63	5413.05	5413.47	5413.89	5414.31

1

		RUNOFF SUMMARY							
		FLOW IN CUBIC FEET PER SECOND							
		TIME IN HOURS, AREA IN SQUARE MILES							
OPERATION		STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	TIME OF MAX STAGE		
HYDROGRAPH AT		P 100	281.	5.70	184.	111.	1.98		
ROUTED TO		REACH1	281.	5.77	182.	110.	1.98		
ROUTED TO		REACH2	281.	5.80	181.	109.	1.98		
ROUTED TO		REACH3	281.	5.87	179.	108.	1.98		
ROUTED TO		REACH4	281.	5.93	177.	106.	1.98		
ROUTED TO		REACH5	281.	6.00	174.	105.	1.98		
ROUTED TO		REACH6	281.	6.07	171.	103.	1.98		
SUMMARY OF KINEMATIC WAVE - MUSKINGUM-Cunge Routing (FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)									
INTERPOLATED TO COMPUTATION INTERVAL									
INSTAQ	ELEMENT	DT	PEAK TIME TO PEAK	VOLUME	DT	COMPUTATION INTERVAL PEAK	VOLUME PEAK		
		(MIN)	(CFS)	(IN)	(MIN)	(CFS)	(IN)		
REACH1	MANE	2.00	280.70	346.00	.86	2.00	280.70	346.00	.86
CONTINUITY SUMMARY (AC-FT) - INFLOW=	.9140E+02	EXCESS=	.0000E+00	OUTFLOW=	.9060E+02	BASIN STORAGE=	.8307E+00	PERCENT ERROR=	.0
REACH2	MANE	1.94	280.70	347.92	.85	2.00	280.70	348.00	.85
CONTINUITY SUMMARY (AC-FT) - INFLOW=	.9031E+02	EXCESS=	.0000E+00	OUTFLOW=	.8987E+02	BASIN STORAGE=	.4308E+00	PERCENT ERROR=	.0
REACH3	MANE	2.00	280.66	352.00	.84	2.00	280.66	352.00	.84
CONTINUITY SUMMARY (AC-FT) - INFLOW=	.9017E+02	EXCESS=	.0000E+00	OUTFLOW=	.8885E+02	BASIN STORAGE=	.1256E+01	PERCENT ERROR=	.1
REACH4	MANE	2.00	280.66	356.00	.83	2.00	280.66	356.00	.83
CONTINUITY SUMMARY (AC-FT) - INFLOW=	.8885E+02	EXCESS=	.0000E+00	OUTFLOW=	.8785E+02	BASIN STORAGE=	.6467E+00	PERCENT ERROR=	.4

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REACH5  MANE      2.00    280.64    360.00    .82      2.00    280.64    360.00    .82
        CONTINUITY SUMMARY (AC-FT) - INFLOW= .8785E+02 EXCESS= .0000E+00 OUTFLOW= .8674E+02 BASIN STORAGE= .9646E+00 PERCENT ERROR=.2
        REACH6  MANE      2.00    280.63    364.00    .81      2.00    280.63    364.00    .81
        CONTINUITY SUMMARY (AC-FT) - INFLOW= .8674E+02 EXCESS= .0000E+00 OUTFLOW= .8525E+02 BASIN STORAGE= .1339E+01 PERCENT ERROR=.2

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*** NORMAL END OF HEC-1 ***

THE PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOL- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLED INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREACK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, LOSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, INFMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

PAGE 1 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1 ID BEAR TRAP CREEK
 2 ID 500 YEAR FLOODPLAIN ASSESSMENT
 3 ID ROUTING USING HEC-1
 4 ID M. RHODES, HYDROMETRICS INC.
 5 ID 13 MAR06

*

* *** TIME SPECIFICATION
 6 IT 2 08APR03 1200 300

*

* *** GLOBAL OUTPUT OPTIONS
 7 IO 4

* *** 500 YR FLOODPLAIN ASSESSMENT

8	KK	P 500						
9	KM	PRECIP						
10	BA	1.98						
11	PH	0.00	0.33	1.12	1.97	2.23	2.58	2.99
	*	***	INFLOW TO CREEK					
	*	***	CLARK S UNIT HYDROGRAPH					
12	UC	0.33	2.88					
13	LS	0	84					
	*	***						

1.4 KK REACH₁
1.5 KM BEAR TRAP CREEK BEGIN TO 25+00 FEET DOWNSTREAM
1.6 RD

17	RC	0.08	0.07	0.08	2500.0	.208	BRTRAP500. OUT
18	RX	0.0	100.0	200.0	203	.205	
19	RY	6360	6340	6300	6298	6300	300
	*	*	*	*	*	6315	400
	*	*	*	*	*	6325	500
	*	*	*	*	*	6370	

20	KK	REACH2	KM	CHANNEL ROUTING	25+00 TO 40+00		
21	KM		RD				
22	RD		RC	0.08	0.07	0.08	
23	RC	0.08	RX	0.0	100	200	.133
24	RX	0.0	RY	5720	5680	5620	.283
25	RY	5720	*	*	*	*	5597
	*	*	*	*	*	*	5600
	*	*	*	*	*	*	5600

26	KK	REACH3	KM	CHANNEL ROUTING	40+00 TO 59+00		
27	KM		RD				
28	RD		RC	0.08	0.07	0.08	
29	RC	0.08	RX	0.0	40.0	44.0	.060
30	RX	0.0	RY	5494	5486	5483	60.0
31	RY	5494	*	*	*	*	5488
	*	*	*	*	*	*	5489
	*	*	*	*	*	*	5490

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
PAGE 2

32	KK	REACH4	KM	CHANNEL ROUTING	59+00 TO 68+00		
33	KM		RD				
34	RD		RC	0.08	0.07	0.08	
35	RC	0.08	RX	0.0	40.0	80.0	.030
36	RX	0.0	RY	5467	5460	5453	120
37	RY	5467	*	*	*	*	5449
	*	*	*	*	*	*	5443
	*	*	*	*	*	*	5441

HEC-1 INPUT

38	KK	REACH5	KM	CHANNEL ROUTING	68+00 TO 78+00		
39	KM		RD				
40	RD		RC	0.08	0.07	0.08	
41	RC	0.08	RX	0.0	40.0	80.0	.030
42	RX	0.0	RY	5432	5429	5425	136
43	RY	5432	*	*	*	*	5422
	*	*	*	*	*	*	5419
	*	*	*	*	*	*	5411

44	KK	REACH6	KM	CHANNEL ROUTING	78+00 TO 92+00		
45	KM		RD				
46	RD		RC	0.08	0.07	0.08	
47	RC	0.08	RX	0.0	20.0	37.0	.030
48	RX	0.0	RY	5416	5412	5410	40.0
49	RY	5416	*	*	*	*	5408
	*	*	*	*	*	*	5409
	*	*	*	*	*	*	5410

44	KK	REACH6	KM	CHANNEL ROUTING	78+00 TO 92+00		
45	KM		RD				
46	RD		RC	0.08	0.07	0.08	
47	RC	0.08	RX	0.0	20.0	37.0	.030
48	RX	0.0	RY	5416	5412	5410	40.0
49	RY	5416	*	*	*	*	5408
	*	*	*	*	*	*	5409
	*	*	*	*	*	*	5410

50 ZZ

* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
* RUN DATE 05JUN06 TIME 09:57:36
* ****

* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
* ****

BEAR TRAP CREEK
500 YEAR FLOODPLAIN ASSESSMENT
ROUTING USING HEC-1
M. RHODES, HYDROMETRICS INC.
113 MARO6

7 TC

OUTPUT	CONTROL	VARIABLES	
IPRINT	4	PRINT CONTROL	
IPLOT	0	PLOT CONTROL	
OSCALE	0.	HYDROGRAPH PLOT SCALE	

HYDROGRAPH TIME DATA

GRAPH TIME DATA	NMIN	IDATE	2 MINUTES IN COMPUTATION INTERVAL
ITIME		8 APR 3	STARTING DATE
		1200	STARTING TIME
NO		300	NUMBER OF HYDROGRAPH ORDINATES
ENDDATE		8 APR 3	ENDING DATE
ENDTIME		2158	ENDING TIME
ICENT		19	CENTURY MARK

COMPUTATION INTERVAL	0.03 HOURS
TOTAL TIME	9.97 HOURS
BASE	
ENGLISH UNITS	
DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECO
STORAGE VOLUME	ACRE-FEET
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

SUBBASIN CHARACTERISTICS

BECOMING DATA

1

SCS LOSS RATE .38 INITIAL ABSTRACTION
STRTL 84.00 CURVE NUMBER
CRVNBR

CLARK UNITGRAPH

19 RY
18 RX

<i>6300.00</i>	<i>6315.00</i>	<i>6325.00</i>	<i>6370.00</i>
<i>205.00</i>	<i>300.00</i>	<i>400.00</i>	<i>500.00</i>

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COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

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WYDROGBA DII BOTTING DATA

NORMAL DEPTH CHANNEL			
ANL	.080	LEFT OVERBANK N-VALUE	
ANCH	.070	MAIN CHANNEL N-VALUE	
ANR	.080	RIGHT OVERBANK N-VALUE	
RINTH	1500.	REACH LENGTH	
SEL	.1330	ENERGY SLOPE	
SEL MAY	.000	MAY BY DAY POD	CHMOD 0644 / CHOMP

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CROSS-SECTION DATA

	LEFT	OVERBANK	---	MAIN	CHANNEL	---	RIGHT	OVERBANK	---
ELEVATION	5720.00	5680.00	5620.00	5600.00	5597.00	5600.00	5610.00	5700.00	
DISTANCE	0.00	100.00	200.00	280.00	283.00	286.00	300.00	400.00	

COMMITTED STOOL AGE - OUTLET QW-ELEVATION DATA

HYDROGRAPH ROUTING DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

NOBMAI: DEPTH CHANNEL

NAME	DEFN	CHANNL	VAL
ANL		CIR	.080
ANCH			.070
ANR			.080
RLNTH			1900.
SEL			.0600
ELMAX			:0

LEFT OVERBANK N-VALUE
MAIN CHANNEL N-VALUE
RIGHT OVERBANK N-VALUE
REACH LENGTH
ENERGY SLOPE
MAX. ELEV. FOR STORAGE

CROSS-SOURCE TUNING DATA

	CROSS SECTION DATA				
	LEFT OVERTANK	MIDDLE	MAIN CHANNEL	RIGHT	OVERTANK
ELEVATION	5494.00	5486.00	5483.00	5486.00	5489.00
DISTANCE	.00	40.00	44.00	48.00	60.00
				120.00	140.00
					160.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA									
STORAGE	.00	.02	.08	.18	.31	.49	.74	.1.16	1.73
OUTFLOW	.00	.82	5.23	15.43	33.24	60.26	89.40	148.05	244.05
ELEVATION	5483.00	5483.58	5484.16	5484.74	5485.32	5485.90	5486.47	5487.05	5487.63
STORAGE	4.10	6.52	9.33	12.51	16.06	19.77	23.55	27.41	31.35
OUTFLOW	533.61	987.80	1664.42	2530.10	3599.11	4895.64	6381.14	8047.70	9889.78
ELEVATION	5488.79	5489.37	5489.95	5490.53	5491.11	5491.69	5492.27	5492.84	5493.42

* REACH4 *
***** CHANNEL ROUTING 59+00 TO 68+00

HYDROGRAPH ROUTING DATA

MUSKINGUM-CUNGE CHANNEL ROUTING

NORMAL DEPTH	CHANNEL	LEFT OVERBANK N-VALUE	MAIN CHANNEL N-VALUE	RIGHT OVERBANK N-VALUE
ANL	.080	.080	.070	.070
ANCH				
ANR	.080	.080	.080	.080
RLNTH	.900.	.900.	.900.	.900.
SEL	.0300	.0300	.0300	.0300
ELMAX	.0	.0	.0	.0
		ENERGY SLOPE	REACH LENGTH	MAX. ELEV. FOR STORAGE/OUTFLOW CALCULATION

	CROSS SECTION LINES			
	LEFT	OVERBANK	MAIN CHANNEL	RIGHT
ELEVATION	5467.00	5460.00	5453.00	5449.00
DISTANCE	.00	40.00	80.00	120.00
			138.00	140.00
				200.00
				320.00

49
48RY
RXELEVATION
DISTANCE
.00
5416.00
20.00
5410.00
37.00
5408.00
40.00
5409.00
44.00
5410.00
64.00
5412.00
100.00
5414.00
120.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	.00	.02	.06	.16	.37	.71	.18	.81	.59
OUTFLOW	.00	.60	.84	.26	.66	.60	.66	.36	.52
ELEVATION	5408.00	5408.42	5408.84	5409.26	5409.68	5410.10	5410.53	5410.95	574.16
STORAGE	4.60	5.76	7.02	8.36	9.78	11.27	12.80	14.35	17.54
OUTFLOW	807.73	1092.07	1422.43	1799.82	2225.43	2727.41	3288.49	3898.57	5262.85
ELEVATION	5412.21	5412.63	5413.05	5413.47	5413.89	5414.31	5414.73	5415.16	5416.00

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND

TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR 6-HOUR	MAXIMUM FLOW FOR 24-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
+	HYDROGRAPH AT P	500	486.	5.67	302.	182.	1.98	
ROUTED TO REACH1		486.	5.73	299.	181.	181.	1.98	
ROUTED TO REACH2		486.	5.77	298.	180.	180.	1.98	
ROUTED TO REACH3		486.	5.87	295.	177.	177.	1.98	
ROUTED TO REACH4		486.	5.90	292.	176.	176.	1.98	
ROUTED TO REACH5		486.	5.97	289.	174.	174.	1.98	
ROUTED TO REACH6		485.	6.03	285.	171.	171.	1.98	

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
REACH1 MANE	(MIN) (CFS)	(MIN)	(IN)	(MIN) (CFS)	(MIN)	(IN)	(IN)	(IN)
REACH2 MANE	2.00	485.91	344.00	1.41	2.00	485.91	344.00	1.41

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1502E+03 EXCESS= .0000E+00 OUTFLOW= .1490E+03 BASIN STORAGE= .1264E+01 PERCENT ERROR= .0

REACH2 MANE 1.94 485.79 345.10 1.40 2.00 485.71 346.00 1.40

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1487E+03 EXCESS= .0000E+00 OUTFLOW= .1481E+03 BASIN STORAGE= .6558E+00 PERCENT ERROR= .0

REACH3 MANE 2.00 485.68 352.00 1.38 2.00 485.68 352.00 1.38

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1484E+03 EXCESS= .0000E+00 OUTFLOW= .1465E+03 BASIN STORAGE= .1789E+01 PERCENT ERROR= .0

BRTRAP500. OUT

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CONTINUITY SUMMARY (AC-FT) - INFLOW= .1465E+03 EXCESS= .0000E+00 OUTFLOW= .1451E+03 BASIN STORAGE= .8975E+00 PERCENT ERROR= .3
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1451E+03 EXCESS= .0000E+00 OUTFLOW= .1436E+03 BASIN STORAGE= .1256E+01 PERCENT ERROR= .2
CONTINUITY SUMMARY (AC-FT) - INFLOW= .1436E+03 EXCESS= .0000E+00 OUTFLOW= .1416E+03 BASIN STORAGE= .1864E+01 PERCENT ERROR= .1
REACH4 MANE      2.00    485.65   354.00    1.37    2.00    485.65   354.00    1.37
REACH5 MANE      2.00    485.54   358.00    1.36    2.00    485.54   358.00    1.36
REACH6 MANE      2.00    485.48   362.00    1.34    2.00    485.48   362.00    1.34

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*** NORMAL END OF HEC-1 ***